

STRUCTURAL Engineering

## STRUCTURAL CALCULATIONS

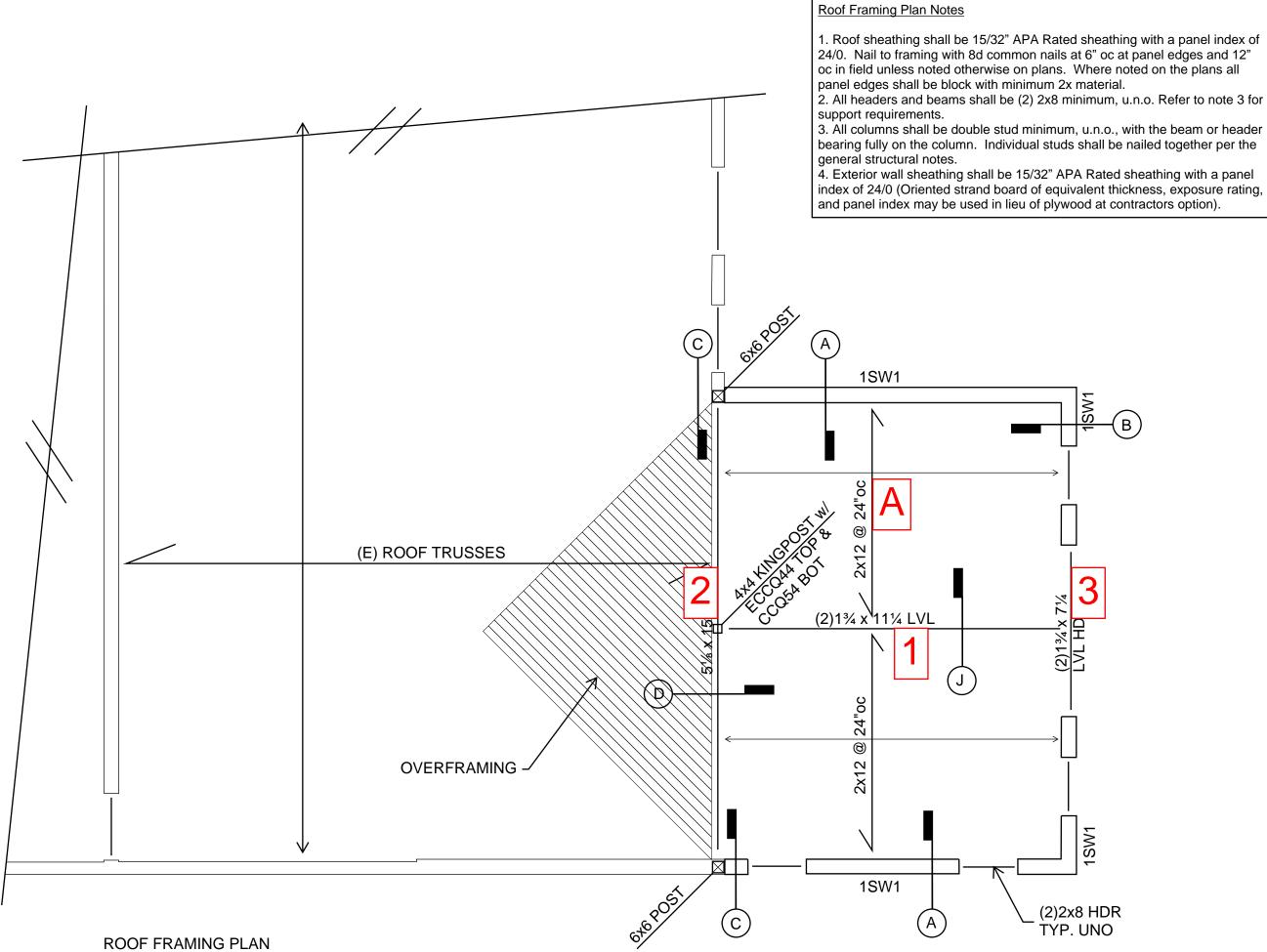
**Christofferson Residence** 4340 90<sup>th</sup> Ave SE Mercer Island, WA 98040

## Beth Christofferson

4340 90<sup>th</sup> Ave SE Mercer Island, WA 98040

March 19, 2024





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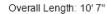
# CHRISTOFFERSON RESIDENCE

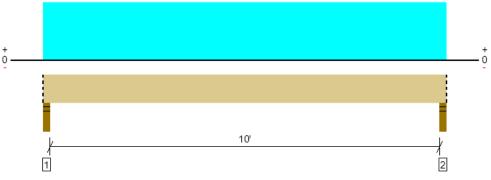
4340 90th AVE SE MERCER ISLAND, WA 98040

## S2.2



### Roof Framing, Joist A 1 piece(s) 2 x 12 HF No.2 @ 24" OC





Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	476 @ 2 1/2"	2126 (3.50")	Passed (22%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	366 @ 1' 2 3/4"	1941	Passed (19%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-Ibs)	1163 @ 5' 3 1/2"	2964	Passed (39%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.052 @ 5' 3 1/2"	0.339	Passed (L/999+)		1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.094 @ 5' 3 1/2"	0.508	Passed (L/999+)		1.0 D + 1.0 S (All Spans)

Member Length : 10' 7" System : Roof Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

• Deflection criteria: LL (L/360) and TL (L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

	Bearing Length			Loads	to Supports		
Supports	Total	Available	Required	Dead	Snow	Factored	Accessories
1 - Stud wall - HF	3.50"	3.50"	1.50"	212	265	476	Blocking
2 - Stud wall - HF	3.50"	3.50"	1.50"	212	265	476	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	10' 7" o/c	
Bottom Edge (Lu)	10' 7" o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Snow	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.15)	Comments
1 - Uniform (PSF)	0 to 10' 7"	24"	20.0	25.0	Default Load

### Weyerhaeuser Notes

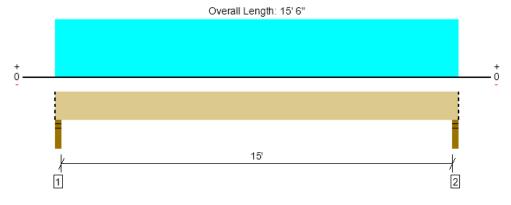
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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3112 @ 1 1/2"	4253 (3.00")	Passed (73%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	2635 @ 1' 2 1/4"	8603	Passed (31%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	11671 @ 7' 9"	18558	Passed (63%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.378 @ 7' 9"	0.508	Passed (L/484)		1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.622 @ 7' 9"	0.762	Passed (L/294)		1.0 D + 1.0 S (All Spans)

Member Length : 15' 6" System : Roof Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

• Deflection criteria: LL (L/360) and TL (L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

	Bearing Length		Loads to Supports (lbs)				
Supports	Total	Available	Required	Dead	Snow	Factored	Accessories
1 - Stud wall - HF	3.00"	3.00"	2.20"	1222	1889	3112	Blocking
2 - Stud wall - HF	3.00"	3.00"	2.20"	1222	1889	3112	Blocking
Blocking Panels are assumed to carry no loa	de annlied di	rectly above t	hem and the	full load is an	nlied to the i	nemher hein	a designed

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 4" o/c	
Bottom Edge (Lu)	15' 6" o/c	
Bottom Edge (Ed)		

•Maximum allowable bracing intervals based on applied load.

			Dead	Snow	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.15)	Comments
0 - Self Weight (PLF)	0 to 15' 6"	N/A	11.5		
1 - Uniform (PSF)	0 to 15' 6" (Front)	9' 9"	15.0	25.0	Default Load

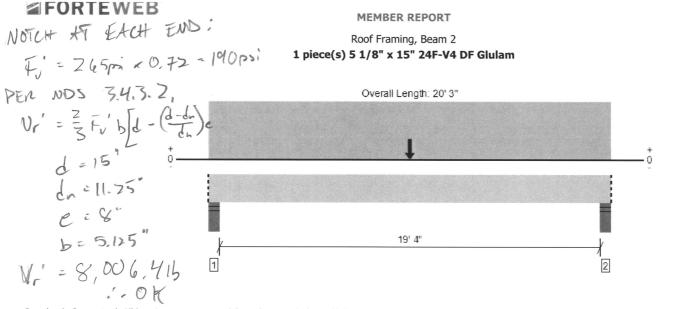
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Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)	5
Member Reaction (lbs)	6503 @ 4"	11416 (5.50")	Passed (57%)		1.0 D + 1.0 S (All Spans)	
Shear (lbs)	5669 @ 1' 8 1/2"	15618	Passed (36%)	1.15	1.0 D + 1.0 S (All Spans)	
Pos Moment (Ft-lbs)	38658 @ 10' 1 1/2"	43531	Passed (89%)	1.15	1.0 D + 1.0 S (All Spans)	
Live Load Defl. (in)	0.572 @ 10' 1 1/2"	0.653	Passed (L/411)		1.0 D + 1.0 S (All Spans)	
Total Load Defl. (in)	0.947 @ 10' 1 1/2"	0.979	Passed (L/248)		1.0 D + 1.0 S (All Spans)	

Aember Length : 20' 3" System : Roof Aember Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Aember Pitch : 0/12

PASSED

• Deflection criteria: LL (L/360) and TL (L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

Critical positive moment adjusted by a volume/size factor of 0.98 that was calculated using length L = 19' 7".

• The effects of positive or negative camber have not been accounted for when calculating deflection.

• The specified glulam is assumed to have its strong laminations at the bottom of the beam. Install with proper side up as indicated by the manufacturer.

Applicable calculations are based on NDS.

	B	Bearing Length			to Support	State of the second second	
Supports	Total	Available	Required	Dead	Snow	Factored	Accessories
1 - Stud wall - HF	5.50"	5.50"	3.13"	2585	3919	6503	Blocking
2 - Stud wall - HF	5.50"	5.50"	3.13"	2585	3919	6503	Blocking

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	20' 3" o/c	
Bottom Edge (Lu)	20' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 20' 3"	N/A	18.7		
1 - Uniform (PSF)	0 to 20' 3" (Front)	11' 9"	15.0	25.0	Default Load
2 - Point (lb)	10' 1 1/2" (Front)	N/A	1222	1889	Linked from: Beam 1, Support 1

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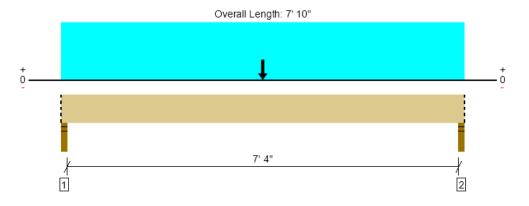
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### 2 piece(s) 1 3/4" x 7 1/4" 2.0E Microllam® LVL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1898 @ 1 1/2"	4253 (3.00")	Passed (45%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	1823 @ 10 1/4"	5544	Passed (33%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	6526 @ 3' 11"	8182	Passed (80%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.168 @ 3' 11"	0.253	Passed (L/542)		1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.279 @ 3' 11"	0.379	Passed (L/327)		1.0 D + 1.0 S (All Spans)

Member Length : 7' 10" System : Roof Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

• Deflection criteria: LL (L/360) and TL (L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

	Bearing Length			Loads	to Support		
Supports	Total	Available	Required	Dead	Snow	Factored	Accessories
1 - Stud wall - HF	3.00"	3.00"	1.50"	757	1140	1898	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	757	1140	1898	Blocking
Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.							

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 10" o/c	
Bottom Edge (Lu)	7' 10" o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Snow	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.15)	Comments
0 - Self Weight (PLF)	0 to 7' 10"	N/A	7.4		
1 - Uniform (PSF)	0 to 7' 10" (Front)	2'	15.0	25.0	Default Load
2 - Point (lb)	3' 11" (Front)	N/A	1222	1889	Linked from: Beam 1, Support 1

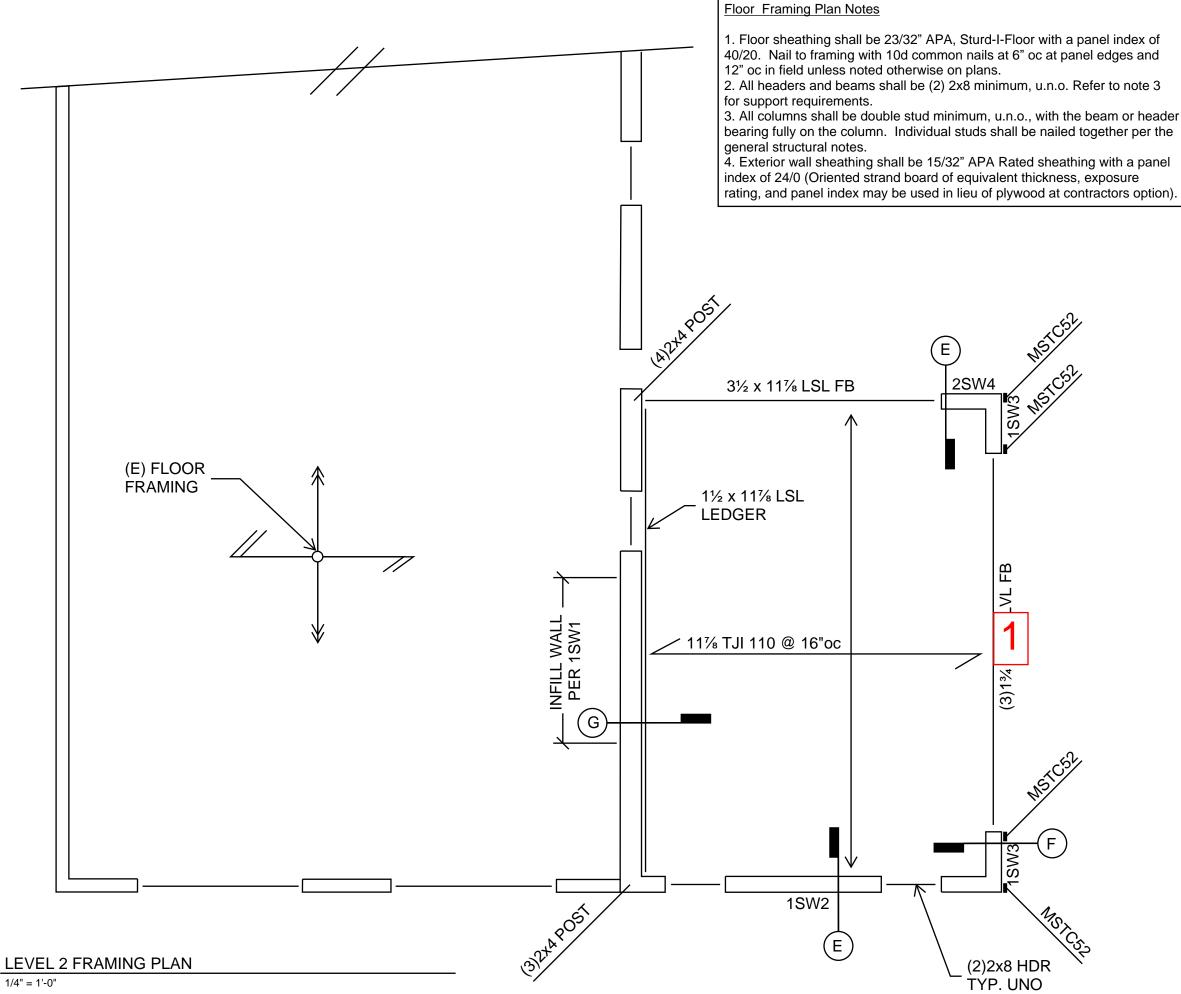
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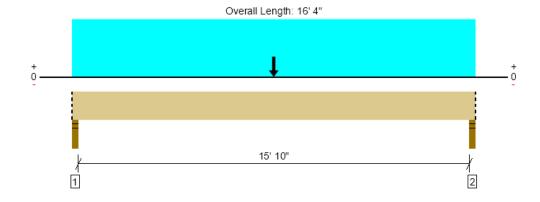
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# CHRISTOFFERSON RESIDENCE

4340 90th AVE SE MERCER ISLAND, WA 98040



### 1 piece(s) 5 1/4" x 11 7/8" 2.0E Parallam® PSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4235 @ 1 1/2"	6379 (3.00")	Passed (66%)		1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3603 @ 1' 2 7/8"	12053	Passed (30%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	22153 @ 8' 2"	34332	Passed (65%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.400 @ 8' 2"	0.402	Passed (L/483)		1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.677 @ 8' 2"	0.804	Passed (L/285)		1.0 D + 0.75 L + 0.75 S (All Spans)

Member Length : 16' 4" System : Floor Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/480) and TL (L/240).

	Bearing Length		Loads to Supports (lbs)					
Supports	Total	Available	Required	Dead	Floor Live	Snow	Factored	Accessories
1 - Stud wall - HF	3.00"	3.00"	1.99"	1689	2450	945	4235	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.99"	1689	2450	945	4235	Blocking
Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.								

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	Continuous	
Bottom Edge (Lu)	Continuous	

			Dead	Floor Live	Snow	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	(1.15)	Comments
0 - Self Weight (PLF)	0 to 16' 4"	N/A	19.5			
1 - Uniform (PSF)	0 to 16' 4" (Front)	7' 6"	15.0	40.0	-	Default Load
2 - Point (lb)	8' 2" (Front)	N/A	1222	-	1889	Linked from: Beam 1, Support 1

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